

Study of pile bearing capacity based on calendering and pile driving analyzer at the faspel development project of Ketek Sikara-kara Mandailing Natal district (North Sumatera, Indonesia)

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Abstract. The deep foundation at the Ketek Sikara-kara wharf is the function of foundation that distribute the load from the upper structure of the building to the soil on the bottom of building to the hard soil layer, without effecting shear failure and settlement, has a bearing capacity allowed. The function of Pile Driving Analyzer (PDA) at Parlindungan Ketek Sikara-kara wharf to determine the bearing capacity of pile and also control to calendering test result, then this researched were tested the pile driving analyzer at 8 sections of pile from 84 sections of calendering pile (10% of the number of sections setting up). Obtained calendering results of lowering is equal to 0.60, 071, 045, 060, 053, 041, 065, 060 fulfilled the specification allowed ≤ 1 inch (2.54 cm). The largest value of pile calendering bearing capacity is 128.52 Ton and the smallest bearing capacity is 73.29 Ton more larger than the allowable pile bearing capacity is 60.9 Ton. Pile driving analyzer results of the largest bearing capacity is equal to TP - B12 Ru 177 Ton more larger than the bearing capacity of pile is allowed on planning is 60.9 Ton and the smallest is equal to TP - B22 Ru 111 Ton more larger than the bearing capacity of pile is allowed on planning is 60.9 Ton.

1. Introduction

The wharf of Parlindungan Ketek is one of the development project of Sikara-kara wharf which taken place shallow up to -2 m LWS due to the location of Sikara-kara wharf is a sediment catchment area that has properties throughout the year experienced sedimentation due to the movement of currents and wave perpendicular to the coast, effecting shallow at the Sikara-kara wharf, the wharf were only docked by ships ≤ 30 GT. In addition, the 2004 tsunami in the Mandailing Natal district caused problems to damage some parts of the Sikara-kara wharf trestle floor. The attendance of a wharf in the Mandailing Natal district is very needed as a production outlet for the area. Potential area Mandailing Natal district one of them has $\pm 100,000$ ha of oil palm plantation is expected to be transported through Sikara-kara wharf.

Before the upper construction was built, there are several tests which were undertaken on the pile foundation, One of testing are loading testing or pre loading, but because of the difficulty of doing pre-loading test in the sea, therefore it were undertaken the test pile driving analyzer. There were two pile test in the field that was calendering test, which was undertaken at each section of pile that were as much as 84 sections in location and pile driving test analyser was undertaken at 8 sections in location.



In this research, all dynamic formulas are empirical formulas, meaning they contain empirical constants whose value depends on the assumptions and conditions at which developed from the results of the monitoring of calculations and investigations performed over a period of time.

Hilley (Modified) on the factors and co-factors that are identified as ambiguous (type and type of hammer / hammer used, method of applying empirical appeal scale to effectiveness reduction factor. Hilley formula actually refers only to the standard of calendering, regarding the effectiveness of Hilley's modified formulas (in developing the relationship between co-earthquake reduction factors, press laterally while the pile, and press while cap pile) (Das, B.M, 2002).

2. Literatur Review

2.1 Calendering

The action of setting up a pile was stopped when it reached hard soil. The indication of setting up pile has reached the hard soil, when the hammer has already set-up on the position. It usually in each tool has already set-up the size of hammer. if it has already set up the hammer in such a position then it was undertaken the reading of calendering immediately. This reading of pile was undertaken on the setting up. If the high reading of hammer value was less than 2.54 cm, then the setting up of pile can already be stopped That means the pile has reached the sections of hard soil that cause of the reading calendering is value small or less 2.54 cm. If the value setting up of pile continued more 2.54 cm then it could be failure. Calendering reading was under take on the last 10 blowing (U.S. Department of Transportation Federal Highway Administration, 2016).

Calendering is a graphic record that available on a tool pile that the function are knowing so far the setting up of pile that has already been undertaken whether has fulfilled the specification of the allowable bearing capacity which was used in working of setting up pile (concrete and steel pipe) to determine the bearing capacity of the soil empirically through the calculations was taken into account by the blowing tool which is a diesel hammer or hydraulic hammer..

Calendering calculations obtain output which is the bearing capacity in Ton. Calendering was undertaken almost approaching the required top pile, final setting for the last 10 blowing ≤ 2.54 cm. One count to know the bearing capacity of calendaring is used Hilley formula (Fellenius,2018) in this research. It is listed below.

Modified Hilley formula for :

Single acting hammer

$$Q_u = \frac{\alpha W H_{ef} \eta}{S + 0.5 (C_c + C_p + C_q)} \quad \text{-----1)}$$

$$Q_r = \frac{Q_u}{3} \quad \text{-----2)}$$

Double acting hammer

$$Q_u = \frac{\alpha E_h \eta}{S + 0.5 ((C_c + C_p + C_q))} \quad \text{-----3)}$$

$$Q_r = \frac{Q_u}{3} \quad \text{-----4)}$$

where :

Q_u = ultimate load capacity of pile at time of driving predicted by driving formula

- Q_r = allowable load
- A = hammer efficiency
- W = weight of ram
- H_{ef} = effective ram stroke
- η = blow Efficiency
- S = permanent set of pile per blow
- P = weight of pile
- E = coefisien of restitution of cushion
- C_c = compression of pile head
- C_p = compression of pile
- C_q = compression of ground

2.2 Pile Driving Analyser (PDA)

Pile Driving Analyser (PDA) was a digital testing to control or determine the bearing capacity of piles has been achieved and also function re-control the results of the Calendaring testing which the function is to stop the pile of each section. Pile driving analyzer test is also a test for dynamically measuring the pile capacity of the deep foundation, it is good for pile or drill pile, pile integrity, and energy from hammer which refers to ASTM D-4945 (*Standard Test Method for High-Strain Dynamic Testing of Deep Foundations*) (Fellenius, 2018).

Analysis of data pile driving analyzer is undertaken by Case Method procedure (Fellenius, 2018), i.e; the data recorded by the PDA are displayed in real time (blow by blow) in the form of wave traces. Routinely, they are also treated analytically and values of stress, energy, etc., are displayed to the operator. The data analysis of pile driving analyzer is undertaken by Case Method procedure, which includes measurement of velocity data and force during re-strike and dynamic variable calculation in real time to get description about bearing capacity of single pile foundation.

Pile driving analyser test, dynamic load test has several advantages, one of it can do multiple pile-test to save time, require relatively small space, evaluate bearing capacity and structural integrity of pile, Evaluate pile settlement. Disadvantages of the pile driving analyzer include: cannot calculate lateral forces, the results can deviate away from test data and interpretation when is undertaken by an inexperienced person.

3. Research Methodology

Development of rehabilitation of Parlimbungan Ketek wharf is suitable for port master plan in port development so that the right port is formed according to its function and rule it refers to Republic Indonesian Law Number. 17 Year 2008 on shipping identifies the need for the provision of port infrastructure as a place of intra and inter moda transportation.

The geographical position of Parlimbungan Ketek wharf is $0^{\circ} 21'47.18''$ LU and $99^{\circ} 7'16.13''$ B.T. The location of Parlimbungan Ketek wharf is located on the west coast of North Sumatera Province and is a strait area directly opposite to Tamang island.

Time schedule for the implementation of the research working was planned within 120 calendar days or 4 months after the warrant starts to commence issues, on time schedule.

The activity objectives of this research was determine whether the capacity of the pile bearing capacity is compatible with the required technical specifications, obtained from the test results of the test pile driving analyzer. Calendaring testing is undertaken to determine if the pile bring down at the last 10 blowing ≤ 2.54 cm and the setting up pile can be stopped and move to the piling sections next.



Figure 1. Location Project Map

Pile which was used in field testing, there is in shown table 1.

Table 1. Project Information & Pile Properties

Table 1.1 Project Information

Data 1	
Pile Name	TP-A 20
DESC	Steel 50- Diesel 6.5 T
File	TP-A 30 PDA
Blow Number	22
Serial Number	4690 LE
Data 2	
File Name	
DESC	Steel 50 – Diesel 6.5 T
File	TP – B 12 PDA
Blow Number	17
Serial Number	4690 LE
Data 3	
File Name	TP A3-3
DESC	Steel 50- Diesel 6.5 T
File	TP A3-3 PDA
Blow Number	5
Serial Number	4523 LE

Table 1.2 Pile Properties

Data 1	
LE	43.7 m
AR	217. 27 cm ²
EM	2109 T/cm ²
SP	7.88 T/ m ³
WS	5123.0 m/s
WC	5.11 m/s
EA/C	89.5 Tn- s/m
2L/C	17.10 ms
JC	0.50
LP	34.9 meter
Data 2	
LE	47.8 meter
AR	213.75 T/m ²
EM	2109 cm ²
SP	7.88 T/m ³
WS	5123.0 m/s
WC	5112.3 m/s
EA/C	88.0 Tn- s/m
2L/C	18.70 ms
JC	0.50
LP	39.5 m
Data 3	
LE	47.5 m

AR	213.75 cm ²
EM	2109 T/cm ²
SP	7.88 T/m ³
WS	5123.0 m/s
WC	5107.5 m/s
EA/C	88.0 Tn-s/m
2L/C	18.50 ms
JC	0.50
LP	39.0 m

4. Result and Discuss

4.1 Result

A. Data 1 : Calendering data of setting up per day

Results which is obtained from the calendaring test for each pile, is obtained bring down of 10 (ten) last blowing as shown in table 2.

Table 2. Calendering data of setting up per day

NO	HARI	TANGGAL	TITIK	KEDALAMAN		Peninggian Pangkal	Jarak antara pangkal tengah	Peninggian ujung atas (selisih beton)	Jumlah Pukul Pukul	Penurunan 3 Pukulan Terakhir (cm)			Rata-rata penurunan 10 Pukulan Terakhir	KET
				(MTR)	Awam					1	2	3		
1	SELASA	29 AGS 2016	A22	48.528	+2.1	0.471	7.40	41.129	400				6.20	
2	SENIN	29 AGS 2016	B22	48.537	+2.1	0.463	7.40	41.137	388				5.90	
3	SENIN	29 AGS 2016	C22	48.538	+2.1	0.468	7.40	41.135	466				7.20	
4	MINGGU	29 AGS 2016	B21	48.445	+2.1	0.515	7.40	41.095	496				7.20	
5	SABTU	29-Sep-16	B21	48.533	+2.1	0.447	7.40	41.153	437				6.30	
6	SABTU	29-Sep-16	B21	48.720	+2.1	0.280	7.40	41.320	477				5.60	
7	MINGGU	29 AGS 2016	C21	48.598	+2.1	0.505	7.40	41.050	488				5.90	
8	MINGGU	29 AGS 2016	A20	47.028	+2.1	0.435	7.40	41.203	433				6.00	
9	MINGGU	29 AGS 2016	B20	48.451	+2.1	0.549	7.40	41.051	474				7.00	
10	JUMAT	30-Sep-16	B20	48.534	+2.1	0.480	7.40	41.134	534				4.90	
11	SENIN	29-Sep-16	C20	48.798	+2.1	0.240	7.40	41.388	487				6.90	
12	JUMAT	30-Sep-16	A19	48.500	+2.1	0.500	7.40	41.200	445				6.20	
13	SELASA	29-Sep-16	B18	48.512	+2.1	0.488	7.40	41.112	448				2.50	
14	BABU	29-Sep-16	B19	48.407	+2.1	0.592	7.40	41.008	509				6.20	
15	SENIN	29-Sep-16	C19	48.408	+2.1	0.600	7.40	41.000	384				5.00	
16	KAMIS	29-Sep-16	B18	48.481	+2.1	0.508	7.40	41.093	435				4.40	
17	KAMIS	29-Sep-16	B18	48.581	+2.1	0.499	7.40	41.093	469				5.00	
18	KAMIS	29-Sep-16	B18	48.570	+2.1	0.490	7.40	41.170	545				3.80	
19	KAMIS	29-Sep-16	C18	48.146	+2.1	0.604	7.40	40.740	400				4.80	
20	JUMAT	30-Sep-16	A17	48.711	+2.1	0.287	7.40	41.313	441				6.90	
21	SABTU	30-Sep-16	B17	48.804	+2.1	0.196	7.40	41.406	383				7.00	
22	MINGGU	30-Sep-16	B17	48.682	+2.1	0.318	7.40	41.282	421				6.90	
23	MINGGU	30-Sep-16	C17	48.558	+2.1	0.442	7.40	41.158	420				5.50	
24	SENIN	29-Sep-16	A16	48.107	+2.1	0.642	7.40	41.058	325				5.60	
25	KAMIS	29-Sep-16	B16	48.590	+2.1	0.410	7.40	41.190	458				6.50	
26	KAMIS	29-Sep-16	B16	48.922	+2.1	0.190	7.40	41.402	539				7.70	
27	KAMIS	29-Sep-16	C16	48.540	+2.1	0.547	7.40	41.050	428				5.50	
28	JUMAT	30-Sep-16	A15	48.779	+2.1	0.221	7.40	41.379	472				5.90	
29	JUMAT	30-Sep-16	B15	48.747	+2.1	0.253	7.40	41.347	454				3.00	
30	SABTU	30-Sep-16	B15	48.258	+2.1	0.742	7.40	41.006	397				5.90	
31	JUMAT	30-Sep-16	C15	48.545	+2.1	0.493	7.50	41.047	384				4.20	
32	SABTU	30-Sep-16	A14	48.555	+2.1	0.445	7.40	41.155	453				7.20	
33	MINGGU	29-Sep-16	B14	49.788	+2.1	0.231	7.50	42.289	476				9.40	
34	SENIN	29-Sep-16	B14	48.533	+2.1	0.467	7.50	41.033	460				2.70	
35	SABTU	29-Sep-16	C14	48.580	+2.1	0.488	7.50	41.020	457				6.60	
36	KAMIS	29-Sep-16	A13	48.778	+2.1	0.222	7.40	41.378	426				6.90	
37	BABU	29-Sep-16	B13	48.481	+2.1	0.519	7.50	41.081	426				7.20	
38	SABTU	1 OKT 2016	B13	48.642	+2.1	0.509	7.50	41.142	384				6.00	
39	SABTU	1 OKT 2016	C13	48.167	+2.1	0.588	7.50	41.112	388				2.40	
40	SABTU	1 OKT 2016	A12	48.198	+2.1	0.200	7.40	41.280	286				5.90	
41	SABTU	1 OKT 2016	B12	48.986	+2.1	0.470	7.50	41.090	469				4.50	
42	SABTU	1 OKT 2016	C12	48.520	+2.1	0.460	7.40	41.120	482				6.00	
43	SELASA	29 AGS 2016	A22	48.528	+2.1	0.471	7.40	41.129	400				6.20	
44	SENIN	29 AGS 2016	A11	48.552	+2.1	0.448	7.50	41.052	382				6.20	
45	SENIN	30 OKT 2016	B11	48.583	+2.1	0.417	7.50	41.083	328				6.90	
46	SENIN	30 OKT 2016	C11	48.674	+2.1	0.329	7.50	41.174	358				7.40	
47	SENIN	30 OKT 2016	A10	48.474	+2.1	0.529	7.50	40.974	313				7.40	
48	SELASA	4 OKT 2016	B10	48.586	+2.1	0.434	7.50	41.086	382				6.60	
49	SENIN	30 OKT 2016	C10	48.478	+2.1	0.524	7.50	40.978	309				7.00	
50	SELASA	4 OKT 2016	A9	48.511	+2.1	0.470	7.50	41.030	383				3.00	
51	SELASA	4 OKT 2016	B9	48.46	+2.1	0.540	7.50	41.060	488				4.00	
52	KAMIS	5 OKT 2016	B9	48.488	+2.1	0.542	7.50	41.058	418				7.30	
53	SELASA	4 OKT 2016	C9	48.514	+2.1	0.488	7.50	41.054	488				6.48	
54	SABTU	8 OKT 2016	A8	48.533	+2.1	0.471	7.50	41.029	403				7.20	
55	KAMIS	6 OKT 2016	B8	48.50	+2.1	0.480	7.50	41.020	385				5.80	
56	SABTU	8 OKT 2016	B8	48.522	+2.1	0.479	7.50	41.022	410				6.00	
57	KAMIS	6 OKT 2016	C8	48.594	+2.1	0.448	7.50	41.054	384				6.00	
58	SABTU	8 OKT 2016	A7	48.697	+2.1	0.350	7.50	41.197	400				9.70	
59	SABTU	8 OKT 2016	B7	48.472	+2.1	0.528	7.50	41.172	344				7.20	
60	SABTU	8 OKT 2016	B7	48.480	+2.1	0.520	7.50	41.060	431				6.90	
61	SABTU	8 OKT 2016	C7	48.688	+2.1	0.331	7.50	41.288	372				6.90	
62	JUMAT	28 OKT 2016	A6	48.504	+2.1	0.498	7.50	41.004	380				4.00	
63	JUMAT	28 OKT 2016	B6	48.588	+2.1	0.451	7.50	41.048	423				4.90	
64	SABTU	29 OKT 2016	B6	48.752	+2.1	0.348	7.50	41.052	405				5.20	
65	JUMAT	28 OKT 2016	C6	48.486	+2.1	0.504	7.50	40.966	381				5.20	
66	SABTU	29 OKT 2016	A5	48.555	+2.1	0.448	7.50	41.055	381				5.60	
67	SABTU	29 OKT 2016	B5	48.680	+2.1	0.320	7.50	41.280	448				6.00	
68	SABTU	29 OKT 2016	B5	48.697	+2.1	0.328	7.50	41.097	427				7.70	
69	SABTU	29 OKT 2016	C5	48.695	+2.1	0.395	7.50	41.105	424				7.50	
70	MINGGU	30 OKT 2016	A4	48.503	+2.1	0.487	7.50	41.008	418				6.90	
71	MINGGU	30 OKT 2016	B4	48.488	+2.1	0.592	7.50	41.008	419				3.80	
72	MINGGU	30 OKT 2016	B4	48.478	+2.1	0.522	7.50	41.078	441				7.00	
73	MINGGU	30 OKT 2016	C4	48.575	+2.1	0.425	7.50	41.075	410				6.50	
74	SENIN	31 OKT 2016	A3	48.693	+2.1	0.347	7.50	41.193	400				5.90	
75	SENIN	31 OKT 2016	B3	48.638	+2.1	0.362	7.50	41.138	406				6.80	
76	BABU	31-Nov-16	B3	48.683	+2.1	0.317	7.50	41.283	427				6.30	
77	SENIN	31 OKT 2016	C3	48.571	+2.1	0.427	7.50	41.073	386				7.30	
78	BABU	31-Nov-16	A2	48.846	+2.1	0.154	7.50	41.346	383				4.80	
79	BABU	31-Nov-16	B2	48.842	+2.1	0.159	7.50	41.342	408				4.18	
80	BABU	31-Nov-16	B2	48.841	+2.1	0.149	7.50	41.341	408				4.10	
81	SELASA	31-Nov-16	C2	48.646	+2.1	0.360	7.50	41.140	419				4.00	
82	SELASA	31-Nov-16	A1	48.762	+2.1	0.238	7.50	41.362	400				4.80	
83	SELASA	31-Nov-16	B1	48.881	+2.1	0.162	7.50	41.381	444				4.80	
84	SABTU	4-Nov-16	C1	48.621	+2.1	0.379	7.50	41.121	459				5.10	

C. Data 4: Recapitulation calendaring of setting up data
 The magnitude of the last 10 (ten) blowing, on the average into Calendaring (final set) as shown table 5.

Table 5. Recapitulation calendaring of setting up data

No	No. Tiang Pancang	Kedalaman TP (Laporan Pemancangan)			Jumlah (m)	Grand Total dan Rerata (m)	Kalendering/ Penurunan (cm)	Penurunan yang diijinkan
		Cut Level ke Muka air (m)	Muka Air ke Sealed (m)	Sealed ke Tanah Keras (m)				
(1)	(2)	(3)	(4)	(5 = 3+4+5)	(7)	(8)		
1	A.1	2,30	5,40	41,262	48,762	145,88	0,56	
2	B.1	2,30	5,40	40,998	48,498	48,63	0,48	
3	C.1	2,30	5,40	41,121	48,621		0,51	
4	A.2	2,30	5,40	41,340	48,640		0,60	
5	B.2	2,30	5,40	42,341	49,641	196,67	0,41	
6	B.2	2,30	5,40	42,041	49,541	49,17	0,41	
7	C.2	2,30	5,40	41,340	48,640		0,60	
8	A.1	2,30	5,40	41,151	48,651		0,53	
9	B.3	2,30	5,40	42,138	49,538	196,55	0,68	
10	B.3	2,30	5,40	42,183	49,683	49,14	0,83	
11	C.3	2,30	5,40	41,073	48,573		0,73	
12	A.4	2,30	5,40	41,003	48,503		0,93	
13	B.4	2,30	5,40	41,908	49,408	195,96	0,38	
14	B.4	2,30	5,40	41,938	49,438	48,99	0,70	
15	C.4	2,30	5,40	41,075	48,575		0,65	
16	A.5	2,30	5,40	41,055	48,555		0,55	
17	B.5	2,30	5,40	42,380	49,680	196,34	0,80	
18	B.5	2,30	5,40	41,997	49,497	49,08	0,77	
19	C.5	2,30	5,40	41,305	48,605		0,75	
20	A.6	2,30	5,40	41,004	48,504		0,60	
21	B.6	2,30	5,40	42,049	49,549	195,90	0,49	
22	B.6	2,30	5,40	41,852	49,352	48,98	0,52	
23	C.6	2,30	5,40	40,996	48,496		0,56	
24	A.7	2,30	5,40	41,197	48,697		0,97	
25	B.7	2,30	5,40	42,172	49,672	196,52	0,72	
26	B.7	2,30	5,40	41,980	49,480	49,13	0,50	
27	C.7	2,30	5,40	41,109	48,609		0,69	
28	A.8	2,30	5,40	41,039	48,539		0,77	
29	B.8	2,30	5,40	42,022	49,522	196,13	0,58	
30	B.8	2,30	5,40	42,039	49,539	49,03	0,90	
31	C.8	2,30	5,40	41,054	48,554		0,60	
32	A.9	2,30	5,40	41,030	48,530		0,80	
33	B.9	2,30	5,40	41,960	49,460	195,96	0,40	
34	B.9	2,30	5,40	41,958	49,458	48,99	0,73	
35	C.9	2,30	5,40	41,034	48,534		0,95	
36	A.10	2,30	5,40	40,974	48,474		0,74	
37	B.10	2,30	5,40	42,166	49,666	196,20	0,66	
38	B.10	2,30	5,40	42,086	49,586	49,05	0,96	
39	C.10	2,30	5,40	40,976	48,476		0,76	
40	A.11	2,30	5,40	41,052	48,552	195,81	0,82	
41	B.11	2,30	5,40	41,083	48,583	48,60	0,83	
42	C.11	2,30	5,40	41,174	48,674		0,74	

No	No. Tiang Pancang	Muka air (m)	Sealed (m)	Tanah Keras (m)	Grand Total dan Rerata (m)	Penurunan (cm)	diijinkan
(1)	(2)	(3)	(4)	(5)	(6 = 3+4+5)	(7)	(8)
1	A.1	2,30	5,40	41,262	48,762	145,88	0,56
43	A.12	2,30	5,30	41,277	48,677	145,77	0,59
44	B.12	2,30	5,40	41,030	48,530	48,57	0,45
45	C.12	2,30	5,30	41,130	48,530		0,60
46	A.13	2,30	5,30	41,378	48,778		0,89
47	B.13	2,30	5,40	41,981	49,481	196,59	0,72
48	B.13	2,30	5,40	42,142	49,642	49,15	0,80
49	C.13	2,30	5,40	41,187	48,687		0,24
50	A.14	2,30	5,30	41,155	48,555		0,72
51	B.14	2,30	5,40	42,269	49,769	196,39	0,94
52	B.14	2,30	5,40	42,033	49,533	49,10	0,37
53	C.14	2,30	5,40	41,032	48,532		0,65
54	A.15	2,30	5,30	41,379	48,779		0,59
55	B.15	2,30	5,30	41,347	49,147	196,33	0,90
56	B.15	2,30	5,30	41,858	49,258	49,08	0,59
57	C.15	2,30	5,40	41,047	48,547		0,42
58	A.16	2,30	5,30	40,957	48,357		0,56
59	B.16	2,30	5,30	42,190	49,590	196,20	0,66
60	B.16	2,30	5,30	42,402	49,802	49,05	0,78
61	C.16	2,30	5,30	41,053	48,453		0,55
62	A.17	2,30	5,30	41,313	48,713		0,70
63	B.17	2,30	5,30	42,404	49,804	196,76	0,71
64	B.17	2,30	5,30	42,282	49,682	49,19	0,81
65	C.17	2,30	5,30	41,158	48,558		0,53
66	A.18	2,30	5,30	41,093	48,493		0,44
67	B.18	2,30	5,30	41,302	48,502	193,71	0,58
68	B.18	2,30	5,30	41,170	48,570	48,43	0,97
69	C.18	2,30	5,30	40,746	48,146		0,49
70	A.19	2,30	5,30	41,100	48,500		0,82
71	B.19	2,30	5,30	42,112	49,512	195,82	0,25
72	B.19	2,30	5,30	42,007	49,407	48,95	0,62
73	C.19	2,30	5,30	41,000	48,400		0,50
74	A.20	2,30	5,30	41,165	48,565		0,60
75	B.20	2,30	5,30	41,051	48,451	195,29	0,20
76	B.20	2,30	5,30	42,114	49,514	48,82	0,49
77	C.20	2,30	5,30	41,358	48,758		0,89
78	A.21	2,30	5,30	41,085	48,485		0,71
79	B.21	2,30	5,30	42,153	49,553	195,81	0,83
80	B.21	2,30	5,30	42,320	49,720	48,95	0,56
81	C.21	2,30	5,30	40,650	48,050		0,53
82	A.22	2,30	5,30	41,129	48,529	145,66	0,61
83	B.22	2,30	5,30	41,137	48,537	48,55	0,37
84	C.22	2,30	5,30	41,192	48,592		0,77

4.2 Discuss

Calculation of bearing capacity of pile using data calendaring and Hilley formula is obtained by bearing capacity of pile as shown in table 6

Table 6. Bearing capacity of calendaring

Pile Name	CALENDERING										Cut water level + water level to seabed + seabed to hard soil (m)	Calendering /settlement (decrease) (cm)
	W Ton	P Ton	H m	S cm	K cm	n	R Ton	EF	SF	R Ton		
TP-A20	604.26	8.36	1.60	0.60	0.95	0.50	370.42	0.90	3.00	111.13	48.565	0.60
TP-A21	604.26	8.36	1.80	0.71	0.90	0.50	401.20	0.90	3.00	120.36	48.485	0.71
TP-B12	604.18	8.36	1.60	0.45	1.90	0.50	244.29	0.90	3.00	73.29	48.530	0.45
TP-C12	604.18	8.36	1.60	0.60	0.95	0.50	370.37	0.90	3.00	111.11	48.510	0.60
TPA3-3	604.18	8.36	1.60	0.53	1.10	0.50	352.19	0.90	3.00	105.66	48.653	0.53
TPB2-2	604.26	8.36	1.60	0.41	1.75	0.50	265.81	0.90	3.00	79.74	49.541	0.41
TP- C4	604.18	8.36	1.60	0.65	1.40	0.50	280.04	0.90	3.00	84.01	48.575	0.65
TP-C8-1	604.18	8.36	1.60	0.60	0.74	0.50	428.42	0.90	3.00	128.52	48.554	0.60

Calculation of bearing capacity of pile using data pile driving analyzer and CAPWAP software, is obtained the bearing capacity of pile as shown in table 7

Table 7. Bearing Capacity of Pile Driving Analyzer

Pile Name	PDA		CAPWAP			
	Bearing Capacity (RMX/RSU) Ton	Bearing Capacity (RU) Ton	Friction (SF) Ton	End Bearing (EB) Ton	Elastic Displacement (Dy) mm	Displacement Maximum (Dx) mm
TP-A20	163	158	157	2	46.39	71.36
TP-A21	125	143	140	3	61.22	97.65
TP-B12	173	177	159	18	16.50	40.37
TP-C12	140	133	125	8.0	37.33	55.19
TP- A3-3	199	174	172	2.0	18.90	28.29
TP- B2-2	111	111	110	1.0	35.22	72.21
TP- C4	177	153	145	8.0	16.31	33.18
TP-C8-1	161	148	148	0.0	11./53	43.49

5. Conclusions

1. The result of the calendering test are 0.60, 071, 045, 060, 053, 041, 065, 060 fulfilled allowable settlement ≤ 1 inchi (2.54 cm).
2. The largest pile bearing capacity calendering is equal to 128.52 ton and the smallest bearing capacity is equal 73.29 Ton > The bearing capacity of pile is allowed on planning is 60.9 Ton
3. CAPWAP software calculation results for Pile Driving Analyzer test at 8 pile sections ie highest bearing capacity on TP - B12 Ru 177 ton > The bearing capacity of pile is allowed on planning is 60.9 Ton and lowest bearing capacity TP - B22 Ru 111 ton > The bearing capacity of pile is allowed on planning is 60.9 Ton
4. The CAPWAP software analysis results for the Pile Driving Analyzer test state that the bearing capacity of the pile is safe to ultimate the load of allowable bearing capacity , Ru > The bearing capacity of pile is allowed on planning is 60.9 Ton

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